



Three-dimensional thermo-elastic analysis and dynamic response of a multi-directional functionally graded skew plate on elastic foundation

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ABSTRACT

Three-dimensional thermo-elastic analysis of a multi-directional functionally graded skew plate on elastic foundation under thermo-mechanical loading is carried out for the first time. Numerical results of displacement and stresses are obtained using differential quadrature method (DQM). Some material properties of the plate assumed to be temperature-dependent and graded in all three spatial directions according to a power law function. The results for various boundary conditions are obtained and the effects of grading index of material properties, temperature distribution, elastic foundation parameters and angle of skew plate are presented. Moreover, the dynamic response of a multi-directional functionally graded material skew plate on elastic foundation is obtained using 4D DQM for the first time. The results show that the material grading direction has a noticeable effect on plate behavior especially for the plates under thermal loading as well as for the dynamic response of the plate.

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1. Introduction

The concept of functionally graded materials (FGMs) was introduced by Japanese researchers at 1984 [1]. FGMs are a new generation of advanced composite materials in which the mechanical properties vary smoothly and continuously in one or more directions [2]. Hence, the composition of several different materials can be used in these structural components in order to optimize the responses of structures subjected to thermal and mechanical loading [3]. The analysis of FGM structures is an interesting and important subject and has attracted the attention of researchers in the last years [4–22].

Plates are well known structures with a wide application in many industries. Rectangular [23–34] and non-rectangular plates [35–37] such as skew plates [38–43] have been widely studied by researches in the literature. Most of researches have used plate theories to analysis the skew plates [39–43] and only few of them used the three-dimensional theory of elasticity. For example Zhou

et al. [38] performed a free vibration analysis for a homogeneous skew plate using three-dimensional elasticity theory.

Several research works (such as [44–48]) have investigated the thermo-elastic analysis of FGM plates. However, thermo-mechanical behavior of FGM skew plates on elastic foundation has been studied rarely.

Joodaky and Joodaky [49] presented an approximate closed-form solution for static behavior of thin skew plates with various boundary conditions rested on Winkler and Pasternak foundations based on elasticity and neutral surface theories of FGMs. The assumed plate was subjected to a uniform load and the Extended Kantorovich Method (EKM) was used together with the idea of weighted residual technique to convert the governing fourth order partial differential equation (PDE) to two ordinary differential equations (ODEs). Lei et al. [50] used the element-free improved moving least-square Ritz (IMLS-Ritz) method to analysis the buckling behavior of functionally graded carbon nanotube (FG-CNT) reinforced composite thick skew plates on Pasternak foundations. The governing equations were derived based on the first-order shear deformation theory (FSDT). Zhang and Liew [51] studied the geometrically nonlinear large deformation analysis of FG-CNT reinforced composite skew plates rested on elastic foundations. They used the FSDT, von Karman

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assumption and the element-free IMLS-Ritz method to derive the governing equations. Katabdari et al. [52] used the energy and Rayleigh-Ritz methods to investigate the free vibration analysis of homogeneous and FGM skew plates rested on variable elastic foundation.

The three-dimensional thermal analysis of a FGM skew plate on elastic foundation has not been considered in the literature. Nevertheless, one of the most common applications of FGMs is in high temperatures where there is a requirement of heat and failure-resistant simultaneously. Based on the best knowledge of the authors, however, there is no work in the literature on multi-directional FGM skew plates.

New applications of FGMs in the environments that temperature varies in two or three directions (such as aerospace shuttles and craft) needs more emphasis on analyses of different structures made of multi-directional FGM such as plates [53,54].

The mechanical behaviors of structures made of multi-directional FGM under different conditions have been investigated by many researchers [55–63]. Thus, various studies were conducted on the analysis of such structures using different methods. Some researchers have used FEM for the mechanical analysis of multi directional FGM plates [53,61]. One important point in using this method is that the stress curve is not continuous over the direction which material changes. A large number of elements are also required in using this method in thermo-mechanical analysis especially in the direction which the material changes. In some studies, analytical or semi-analytical methods were used [62,63]. Several simplifying assumptions on boundary conditions, load and material distribution cause the results to be valid only under specific conditions.

The DQM is a powerful numerical method used by many researchers to analyze the different structures in the last years [64–75]. Fast convergence and accuracy of results are the most benefits of this method. On the other hand, many of papers published in the field of FG plates are based on plate theories [76] which is not very accurate for thick plates. Hence, effective numerical techniques for three dimensional analyses of FG structures can provide more accurate results as the authors [77] used the DQM to thermo-mechanical analysis of a multi-directional FGM plate based on theory of elasticity.

To the best of authors' knowledge, there is no result for the three dimensional thermo-mechanical analysis of FG skew plates on elastic foundation. Moreover, no study has been performed to analysis the multi-directional FG skew plates and there is no paper deals with dynamic response of a FG skew plate on elastic foundation.

In the current study, the three-dimensional conduction heat transfer equation is employed to estimate the temperature distribution in the skew plate. Afterwards, the temperature distribution determined in each node is used to obtain the displacement and stress distributions in the plate. Moreover, 4D DQM is used to obtain the dynamic response of FGM skew plates for the first time. The effect of different parameters on the response of skew FGM plates on elastic foundation is studied.

2. Material distributions

A skew 3D-FGM plate of length a , width b and thickness h are considered. Schematic view of the plate and the directions of η , ζ and z are illustrated in Fig. 1. If θ equals to zero, skew plate is converted to a rectangular plate. The temperature difference between the outer surfaces leads to a temperature gradient and consequently thermal stresses on plate. Young's modulus, Poisson's

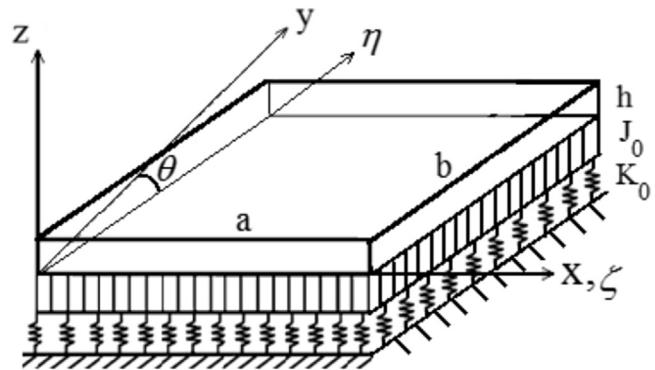


Fig. 1. 3D-FGM skew plate on elastic foundation.

ratio, density and coefficient of thermal expansion are assumed to be graded in all three spatial coordinates (η , ζ , z). The 3D-FGM skew plate is made of two constituent materials and material distribution for the plate is introduced in Eq. (1)

$$P = [P_1 - P_2]V_{\eta\zeta z} + P_2 \quad (1-1)$$

$$V_{\eta\zeta z} = \left(\frac{4\eta}{a} \left(1 - \frac{\eta}{a} \right) \right)^{n_\eta} \left(\frac{4\zeta}{b} \left(1 - \frac{\zeta}{b} \right) \right)^{n_\zeta} \left(\frac{z}{h} \right)^{n_z} \quad (1-2)$$

where P_1 and P_2 are material parameters such as a Young's modulus and P is the property of multi-directional FGM plate. n_η , n_ζ and n_z are volume fraction exponents at η , ζ and z directions with non-negative values. By setting one, two or three volume fraction exponents equal to zero, proper equations for 2D-FGM and 1D-FGM or isotropic plate are made, respectively.

Similar to [78], all material properties are considered to be temperature-dependent excluding thermal conductivity. Considering thermal conductivities as temperature dependent parameters makes the problem nonlinear. Temperature dependencies of material parameters (e.g. Q) is considered as following [78].

$$Q(T) = Q_0 \left(Q_{-1} T^{-1} + 1 + Q_1 T + Q_2 T^2 + Q_3 T^3 \right) \quad (2)$$

$Q_i (i = -1, 0, 1, 2, 3)$ are constants and dependent on the material.

3. Governing equations

The solution process consists of two main steps, "thermal analysis" and "mechanical analysis". The solution method is uncoupled and firstly, the steady state heat transfer analysis is performed to obtain the temperature distribution due to temperature difference between the outer surfaces of plate and then the mechanical analysis is achieved to obtain the displacements and stresses. Governing equations is derived in Cartesian coordinates (x, y, z) and then a proper variable change are used to obtain the equations for skew plate coordinates (η, ζ, z).

3.1. Thermal analysis

Three dimensional steady state heat transfer equation in absence of generation resources is as follows

$$\frac{\partial}{\partial x} \left[K_x \frac{\partial T}{\partial x} \right] + \frac{\partial}{\partial y} \left[K_y \frac{\partial T}{\partial y} \right] + \frac{\partial}{\partial z} \left[K_z \frac{\partial T}{\partial z} \right] = 0 \quad (3-1)$$

in which K is the thermal conductivity. The boundary conditions are related to temperature values of outer surfaces of the plate as

$$T(x, y, 0) = T(0, y, z) = T(a, y, z) = T(x, 0, z) = T(x, b, z) = T_b \quad (3-2)$$

and

$$T(x, y, h) = 300 + (T_c - 300) \times \sin\left(\frac{\pi}{a}x\right) \sin\left(\frac{\pi}{b}y\right) \quad (3-3)$$

where T_b and T_c are constant values.

3.2. Mechanical analysis

The stress-strain relations in thermo-elastic constitutive equations can be written as follows [79].

$$\sigma_x = \frac{E}{(1+\nu)(1-2\nu)} [(1-\nu)\epsilon_x + \nu(\epsilon_y + \epsilon_z)] - \frac{E}{1-2\nu} \int_{T_0}^T \alpha dT \quad (4-1)$$

$$\sigma_y = \frac{E}{(1+\nu)(1-2\nu)} [(1-\nu)\epsilon_y + \nu(\epsilon_x + \epsilon_z)] - \frac{E}{1-2\nu} \int_{T_0}^T \alpha dT \quad (4-2)$$

$$\sigma_z = \frac{E}{(1+\nu)(1-2\nu)} [(1-\nu)\epsilon_z + \nu(\epsilon_x + \epsilon_y)] - \frac{E}{1-2\nu} \int_{T_0}^T \alpha dT \quad (4-3)$$

$$\tau_{xy} = \frac{E}{(1+\nu)} \epsilon_{xy}, \tau_{xz} = \frac{E}{(1+\nu)} \epsilon_{xz}, \tau_{yz} = \frac{E}{(1+\nu)} \epsilon_{yz} \quad (4-4)$$

in which T_0 is the initial temperature and T is the current temperature. The linear strain-displacement relations are defined as

$$\begin{aligned} \epsilon_x &= \frac{\partial u}{\partial x}, \epsilon_y = \frac{\partial v}{\partial y}, \epsilon_z = \frac{\partial w}{\partial z} \\ \epsilon_{xy} &= \frac{1}{2} \left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right), \epsilon_{xz} = \frac{1}{2} \left(\frac{\partial u}{\partial z} + \frac{\partial w}{\partial x} \right), \epsilon_{yz} = \frac{1}{2} \left(\frac{\partial v}{\partial z} + \frac{\partial w}{\partial y} \right) \end{aligned} \quad (5)$$

where u, v and w are displacement components at x, y and z directions, respectively. Equations of motion in absence of body forces are

$$\frac{\partial \sigma_x}{\partial x} + \frac{\partial \tau_{xy}}{\partial y} + \frac{\partial \tau_{xz}}{\partial z} = \rho \frac{\partial^2 u}{\partial t^2} \quad (6-1)$$

$$\frac{\partial \tau_{xy}}{\partial x} + \frac{\partial \sigma_y}{\partial y} + \frac{\partial \tau_{yz}}{\partial z} = \rho \frac{\partial^2 v}{\partial t^2} \quad (6-2)$$

$$\frac{\partial \tau_{xz}}{\partial x} + \frac{\partial \tau_{yz}}{\partial y} + \frac{\partial \sigma_z}{\partial z} = \rho \frac{\partial^2 w}{\partial t^2} \quad (6-3)$$

Combining Eq. (5) with Eq. (4) and inserting the result into Eq. (6), gives the equations of motion for three directional FGM plate in terms of displacement components as

$$\begin{aligned} &\left(\frac{\frac{\partial E}{\partial x}(1+\nu)(1-2\nu) - E \left(\frac{\partial \nu}{\partial x}(1-2\nu) - 2 \frac{\partial \nu}{\partial x}(1+\nu) \right)}{(1+\nu)^2(1-2\nu)^2} \right) \left[(1-\nu) \frac{\partial u}{\partial x} + \nu \left(\frac{\partial v}{\partial y} + \frac{\partial w}{\partial z} \right) \right] \\ &+ \frac{E}{(1+\nu)(1-2\nu)} \left[- \frac{\partial v}{\partial x} \frac{\partial u}{\partial x} + (1-\nu) \frac{\partial^2 u}{\partial x^2} + \frac{\partial \nu}{\partial x} \left(\frac{\partial v}{\partial y} + \frac{\partial w}{\partial z} \right) + \nu \left(\frac{\partial^2 v}{\partial x \partial y} + \frac{\partial^2 w}{\partial x \partial z} \right) \right] \\ &- \frac{\frac{\partial E}{\partial x}(1-2\nu) + 2E \frac{\partial \nu}{\partial x}}{(1-2\nu)^2} \int_{T_0}^T \alpha dT - \frac{E}{(1-2\nu)} \frac{\partial}{\partial x} \left(\int_{T_0}^T \alpha dT \right) \\ &+ \frac{1}{2} \frac{\partial E}{\partial y} \frac{(1+\nu) - E \frac{\partial \nu}{\partial y}}{(1+\nu)^2} \left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) + \frac{1}{2} \frac{E}{(1+\nu)} \left(\frac{\partial^2 u}{\partial y^2} + \frac{\partial^2 v}{\partial x \partial y} \right) \\ &+ \frac{1}{2} \frac{\partial E}{\partial z} \frac{(1+\nu) - E \frac{\partial \nu}{\partial z}}{(1+\nu)^2} \left(\frac{\partial w}{\partial z} + \frac{\partial u}{\partial x} \right) + \frac{1}{2} \frac{E}{(1+\nu)} \left(\frac{\partial^2 w}{\partial x \partial z} + \frac{\partial^2 u}{\partial z^2} \right) = \rho \frac{\partial^2 u}{\partial t^2} \end{aligned} \quad (7-1)$$

In Eqs. (7-1) to (7-3), all properties of material (such as E and ν) in all nodes of the domain are calculated based on Eqs. (1) and (2). The boundary conditions used in this paper are defined as follows

$$\begin{aligned}
& \frac{1}{2} \frac{\partial E}{\partial x} (1 + \nu) - E \frac{\partial \nu}{\partial x} \left(\frac{\partial u}{\partial y} + \frac{\partial \nu}{\partial x} \right) + \frac{1}{2} \frac{E}{(1 + \nu)} \left(\frac{\partial^2 u}{\partial x \partial y} + \frac{\partial^2 \nu}{\partial x^2} \right) \\
& + \left(\frac{\frac{\partial E}{\partial y} (1 + \nu) (1 - 2\nu) - E \left(\frac{\partial \nu}{\partial y} (1 - 2\nu) - 2 \frac{\partial \nu}{\partial y} (1 + \nu) \right)}{(1 + \nu)^2 (1 - 2\nu)^2} \right) \left[(1 - \nu) \frac{\partial \nu}{\partial y} + \nu \left(\frac{\partial u}{\partial x} + \frac{\partial w}{\partial z} \right) \right] \\
& + \frac{E}{(1 + \nu)(1 - 2\nu)} \left[- \frac{\partial \nu}{\partial y} \frac{\partial \nu}{\partial y} + (1 - \nu) \frac{\partial^2 \nu}{\partial y^2} + \frac{\partial \nu}{\partial y} \left(\frac{\partial u}{\partial x} + \frac{\partial w}{\partial z} \right) + \nu \left(\frac{\partial^2 u}{\partial x \partial y} + \frac{\partial^2 w}{\partial y \partial z} \right) \right] \quad (7-2)
\end{aligned}$$

$$\begin{aligned}
& \frac{\partial E}{\partial y} (1 - 2\nu) + 2E \frac{\partial \nu}{\partial y} \int_{T_0}^T \alpha dT - \frac{E}{(1 - 2\nu)} \frac{\partial \left(\int_{T_0}^T \alpha dT \right)}{\partial y} \\
& + \frac{1}{2} \frac{\partial E}{\partial z} (1 + \nu) - E \frac{\partial \nu}{\partial z} \left(\frac{\partial \nu}{\partial z} + \frac{\partial w}{\partial y} \right) + \frac{1}{2} \frac{E}{(1 + \nu)} \left(\frac{\partial^2 \nu}{\partial z^2} + \frac{\partial^2 w}{\partial y \partial z} \right) = \rho \frac{\partial^2 \nu}{\partial t^2}
\end{aligned}$$

$$\begin{aligned}
& \frac{1}{2} \frac{\partial E}{\partial x} (1 + \nu) - E \frac{\partial \nu}{\partial x} \left(\frac{\partial u}{\partial z} + \frac{\partial w}{\partial x} \right) + \frac{1}{2} \frac{E}{(1 + \nu)} \left(\frac{\partial^2 u}{\partial x \partial z} + \frac{\partial^2 w}{\partial x^2} \right) \\
& + \frac{1}{2} \frac{\partial E}{\partial y} (1 + \nu) - E \frac{\partial \nu}{\partial y} \left(\frac{\partial \nu}{\partial z} + \frac{\partial w}{\partial y} \right) + \frac{1}{2} \frac{E}{(1 + \nu)} \left(\frac{\partial^2 \nu}{\partial y \partial z} + \frac{\partial^2 w}{\partial y^2} \right) \\
& + \left(\frac{\frac{\partial E}{\partial z} (1 + \nu) (1 - 2\nu) - E \left(\frac{\partial \nu}{\partial z} (1 - 2\nu) - 2 \frac{\partial \nu}{\partial z} (1 + \nu) \right)}{(1 + \nu)^2 (1 - 2\nu)^2} \right) \left[(1 - \nu) \frac{\partial w}{\partial z} + \nu \left(\frac{\partial u}{\partial x} + \frac{\partial v}{\partial y} \right) \right] \\
& + \frac{E}{(1 + \nu)(1 - 2\nu)} \left[- \frac{\partial \nu}{\partial z} \frac{\partial w}{\partial z} + (1 - \nu) \frac{\partial^2 w}{\partial z^2} + \frac{\partial \nu}{\partial z} \left(\frac{\partial u}{\partial x} + \frac{\partial v}{\partial y} \right) + \nu \left(\frac{\partial^2 u}{\partial x \partial z} + \frac{\partial^2 v}{\partial y \partial z} \right) \right] \\
& - \frac{\partial E}{\partial z} (1 - 2\nu) + 2E \frac{\partial \nu}{\partial z} \int_{T_0}^T \alpha dT - \frac{E}{(1 - 2\nu)} \frac{\partial \left(\int_{T_0}^T \alpha dT \right)}{\partial z} = \rho \frac{\partial^2 w}{\partial t^2} \quad (7-3)
\end{aligned}$$

$$\text{SSSS } \begin{cases} \zeta = 0, a \rightarrow \nu_\eta = w = \sigma_n = 0 \\ \eta = 0, b \rightarrow u_\zeta = w = \sigma_n = 0 \end{cases} \quad (8-1)$$

$$\sigma_z = q, \tau_{xz} = \tau_{yz} = 0 \text{ at } z = h \quad (8-5)$$

$$\text{CCCC } \begin{cases} \zeta = 0, a \rightarrow u_\zeta = 0, \nu_\eta = 0, w = 0 \\ \eta = 0, b \rightarrow u_\zeta = \nu_\eta = w = 0 \end{cases} \quad (8-2)$$

$$\sigma_z = K_w w - K_{sx} \frac{\partial^2 w}{\partial x^2} - K_{sy} \frac{\partial^2 w}{\partial y^2}, \tau_{xz} = \tau_{yz} = 0 \text{ at } z = 0 \quad (8-6)$$

$$\text{SCSC } \begin{cases} S : \zeta = 0, a \rightarrow \nu_\eta = w = \sigma_n = 0 \\ C : \eta = 0, b \rightarrow u_\zeta = \nu_\eta = w = 0 \end{cases} \quad (8-3)$$

where q is the mechanical loading and can vary through x and y directions and also $K_w = \frac{E_0 K_0 h^3}{d^4}$, $K_{sx} = \frac{\nu_0 E_0 J_0 h^3}{d^2}$, $K_{sy} = \frac{\nu_0 E_0 J_0 h^3}{b^2}$ in which $\nu_0 = 0.3$ and $E_0 = 10^9$ Pa and boundary conditions in time domain is

$$u = v = w = 0 \text{ and } \frac{\partial u}{\partial t} = \frac{\partial v}{\partial t} = \frac{\partial w}{\partial t} = 0 \text{ at } t = 0 \quad (8-7)$$

For using Eqs. (7) and (8) in the skew plate the variable changes in appendix. 1 are applied.

$$\text{FCFC } \begin{cases} F : \zeta = 0, a \rightarrow \sigma_n = \tau_{\zeta\eta} = \tau_{\zeta z} = 0 \\ C : \eta = 0, b \rightarrow u_\zeta = \nu_\eta = w = 0 \end{cases} \quad (8-4)$$

in which σ_n is the normal stress and u_ζ and ν_η are defined in appendix. 1. Boundary conditions of the top and bottom surfaces of the plate are

4. Differential quadrature method (DQM)

The equations obtained in previous section are solved by a version of DQM named GDQ [11,80–82]. Relation between m^{th} time derivation of function related to x and function values in all N points of the domain is

$$\frac{d^m f(x)}{dx^m} \Big|_{x=x_i} = \sum_{j=1}^N C_{ij}^{(m)} f(x_j), \quad i = 1, 2, \dots, N \quad (9)$$

in which the weight coefficients for the first time and higher order derivations until $N-1$ are

$$C_{ij}^{(1)} = \frac{\prod_{j=1, j \neq i}^N (x_i - x_j)}{(x_i - x_k) \prod_{j=1, j \neq k}^N (x_k - x_j)}, \quad i, j, k = 1, 2, \dots, N \quad (10-1)$$

$$C_{ii}^{(1)} = - \sum_{j=1, j \neq i}^N C_{ij}^{(1)}, \quad i = 1, 2, \dots, N \quad (10-2)$$

$$C_{ij}^{(m)} = m \left[C_{ii}^{(m-1)} C_{ij}^{(1)} - \frac{C_{ij}^{(m-1)}}{x_i - x_j} \right], \quad i, j = 1, 2, \dots, N \quad (10-3)$$

$$C_{ii}^{(m)} = - \sum_{j=1, j \neq i}^N C_{ij}^{(m)}, \quad i = 1, 2, \dots, N \quad (10-4)$$

where x_i (node points) is obtained from Chebyshev polynomials (l is domain length) as

$$x_i = 0.5l \left(1 - \cos \left(\frac{(i-1) \times \pi}{N-1} \right) \right) \quad (11)$$

Both space and time domains are discretized based on Eq. (11). Moreover, Eq. (9) is used for all derivatives related to x, y, z and t in Eq. (7). The in-plane grid of skew plate is shown in Fig. 2.

5. Validations

To validate the results, the following comparisons with published data are performed.

5.1. Case study for mechanical analysis of a skew plate

Consider a fully clamped homogenous skew plate

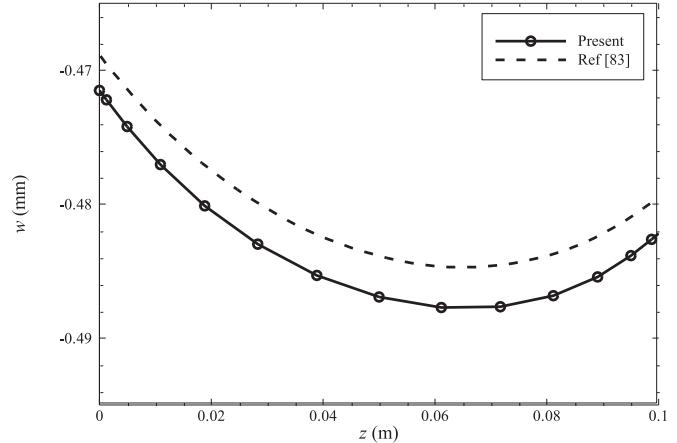


Fig. 3. Through the thickness variation of deflection of a homogenous skew plate.

with the geometry and material properties as $a = 0.5\text{m}$, $b = 1\text{m}$, $h = 0.1\text{m}$, $E = 70\text{GPa}$, $\nu = 0.3$, $\theta = 30^\circ$. The plate is subjected to a uniform pressure loading $q = 20 \text{ MPa}$ on the top surface. Through the thickness distribution of transverse deflection are shown in Fig. 3. A good agreement between the obtained results and Ref [83] can be seen and the difference between two curves is less than 0.8%. Moreover a convergence study is shown in Table 1.

5.2. Case study for thermo-mechanical analysis of a FGM rectangular plate

Consider a FGM square plate ($a = b = 10h$) with the following materials properties

$$\begin{aligned} \text{Monel : } K'_1 &= 227.24 \text{ GPa}, G_1 = 65.55 \text{ GPa}, \alpha_1 \\ &= 15 \times 10^{-6} / \text{K}, K_1 = 25 \text{ W/mK} \end{aligned}$$

$$\begin{aligned} \text{Zirconia : } K'_2 &= 125.83 \text{ GPa}, G_2 = 58.077 \text{ GPa}, \alpha_2 \\ &= 10 \times 10^{-6} / \text{K}, K_2 = 2.09 \text{ W/mK} \end{aligned}$$

where K' , G , α and K denote bulk modulus, shear modulus, thermal expansion coefficient and thermal conductivity, respectively. The local effective material properties based on Mori-Tanaka estimates are:

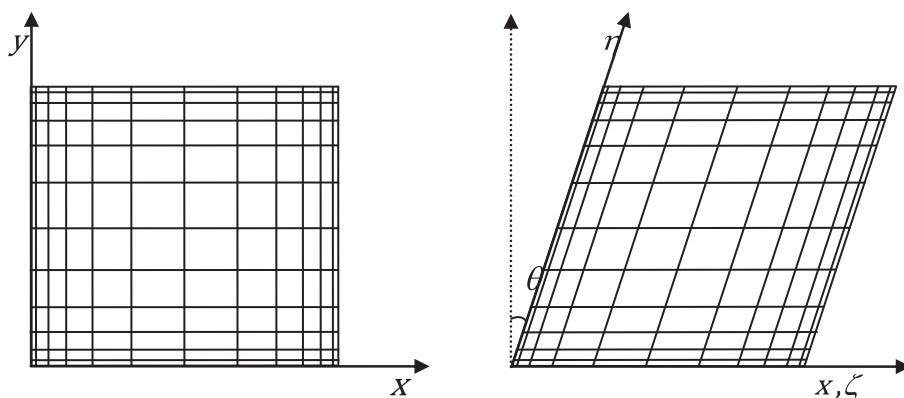


Fig. 2. A schematic in-plane grid used for skew plate.

Table 1

Convergence study of bending analysis of a skew plate.

$N_x \times N_y \times N_z$	$w(a/2, b/2, h/2)$ (mm)	$\sigma_x(a/2, b/2, 0)$ (MPa)	$\tau_{xy}(a/2, b/2, 0)$ (MPa)
5 × 5 × 5	-0.51324	84.83872	-25.4022
7 × 7 × 7	-0.49438	85.62554	-28.3109
9 × 9 × 9	-0.48311	80.0829	-28.497
11 × 11 × 11	-0.48797	82.49071	-28.5791
13 × 13 × 13	-0.48723	81.74038	-28.1526
15 × 15 × 15	-0.48747	82.5489	-28.4798
17 × 17 × 17	-0.48732	81.72077	-28.2284
19 × 19 × 19	-0.4874	82.51728	-28.4833

$$\begin{aligned} \frac{K' - K'_1}{K'_2 - K'_1} &= \frac{V_2}{1 + (1 - V_2)(K'_2 - K')_1 / \left(K'_1 + \frac{4}{3}G_1 \right)}, \quad V_2 = (z/h)^{n_z} \\ \frac{G - G_1}{G_2 - G_1} &= \frac{V_2}{1 + (1 - V_2)(G_2 - G_1)/(G_1 + f_1)}, \quad f_1 = \frac{G_1(9K'_1 + 8G_1)}{6(K'_1 + 2G_1)} \\ \frac{K - K_1}{K_2 - K_1} &= \frac{V_2}{1 + (1 - V_2)(K_2 - K_1)/3K_1} \\ \frac{\alpha - \alpha_1}{\alpha_2 - \alpha_1} &= \frac{\frac{1}{K'} - \frac{1}{K'_1}}{\frac{1}{K'_2} - \frac{1}{K'_1}} \end{aligned} \quad (12)$$

Mechanical boundary conditions are SSSS with the thermal boundary conditions as:

$$\begin{aligned} \sigma_x^* &= -\frac{h^2}{qa^2}\sigma_x\left(\frac{a}{2}, \frac{b}{2}, 0\right), \quad \sigma_y^* = -\frac{h^2}{qa^2}\sigma_y\left(\frac{a}{2}, \frac{b}{2}, 0\right), \quad \sigma_{xy}^* = -\frac{h^2}{qa^2}\sigma_{xy}(0, 0, 0) \\ w^* &= \frac{100D_0}{qa^4}w\left(\frac{a}{2}, \frac{b}{2}, \frac{h}{2}\right), \quad D_0 = \frac{Ech^3}{12(1-\nu^2)}, \quad K_0 = \frac{K_w a^4}{E_0 h^3}, \quad J_0 = \frac{K_{sx} a^2 \nu}{E_0 h^3} = \frac{K_{sy} b^2 \nu}{E_0 h^3} \end{aligned} \quad (14)$$

$$\begin{aligned} T(x, y, h) &= T^+ \sin\left(\frac{m_1 \pi}{a}x\right) \sin\left(\frac{m_2 \pi}{b}y\right) \\ T(x, y, 0) &= T(0, y, z) = T(a, y, z) = T(x, 0, z) = T(x, b, z) = 0 \end{aligned} \quad (13)$$

Table 2 shows a comparison of non-dimensional deflection and normal stresses ($\bar{w} = \frac{w}{10^{-6}T^+a}$, $\bar{\sigma}_{ij} = \frac{\sigma_{ij}}{10^3T^+}$) with those reported in Ref. [84].

Table 2Results for FGM square plate under thermal load ($m_1 = m_2 = 1, a = b, a/h = 10, n_x = n_y = 0, n_z = 2$).

z/h	Ref. [84]			
	11 × 11 × 11	13 × 13 × 13	15 × 15 × 15	17 × 17 × 17
\bar{w}	5.4035	5.8369	5.9633	6.0016
0.5	5.0104	5.449	5.5769	5.6156
0	4.9039	5.3375	5.464	5.5023
$\bar{\sigma}_x$	1 -1072.3	-1025.8	-1012.2	-1008.1
0.5	-244.5	-243.9	-243	-243
0	2.9	-52.5	-68.7	-73.6
$\bar{\sigma}_z$	0.5 4.8555	0.8918	1.3846	1.0035
				1.015

5.3. Case studies for the homogeneous and FGM rectangular plates on elastic foundation

5.3.1. Homogeneous plate

A homogeneous square thin plate ($a = b = 100h$) on Winkler-Pasternak foundation is considered. The plate is subjected to a uniform load on its top surface. The obtained results for central deflection of the plate for three different boundary conditions (SSSS, SCSC and SFSF) and various foundation parameters are shown in **Table 3**. The non-dimensional foundation parameters k_w and k_p are defined as:

$$k_w = \frac{K_w a^4}{D}, \quad k_p = \frac{K_p a^2}{D} \text{ where } D = Eh^3 / 12(1 - \nu^2) \text{ and } K_p = K_{sx} = K_{sy}$$

Comparing these results with those reported by Huang et al. [85] and Lam et al. [86] shows a good agreement.

5.3.2. FGM plate

A simply supported FGM rectangular plate consists of aluminum and alumina is considered. The Young's modulus and Poisson's ratio of these materials are:

$$E_m = 70 \text{ GPa}, \quad \nu_m = 0.3 \text{ Aluminum :}$$

$$E_c = 380 \text{ GPa}, \quad \nu_c = 0.3 \text{ Alumina :}$$

Deflection and stress at the middle point of the plate is shown in **Table 4**. The parameters used here are as following with $E_0 = 1.0 \text{ GPa}, \nu = 0.3$:

5.4. Case study to validate the dynamic analysis

A simply supported square plate of side length $a = 2 \text{ m}$ and thickness $h = 0.01 \text{ m}$ made of aluminum is considered with the Young's modulus, Poisson's ratio and density of $E = 70 \text{ GPa}, \nu = 0.3$ and $\rho = 2707 \text{ kg/m}^3$, respectively. A 4D DQM ($N_x \times N_y \times N_z \times N_t$) is used and **Fig. 4** shows the results for the evaluation of center deflection of plate under a suddenly applied uniform load of intensity $q_0 = 1 \text{ MPa}$ and its comparison with Refs. [88,89]. The non-dimensional parameters used here are:

$$w^* = 70 \times 10^9 wh / (q_0 a^2), \quad t^* = t \sqrt{70 \times 10^9 / (2707 a^2)} \quad (15)$$

A convergence study for this problem is shown in **Fig. 5**.

6. Numerical results and discussions

Consider a thick FGM skew plate of side equal to $a = b = 1 \text{ m}$. Material distribution is according to Eq. (1) and the constituent materials are SUS304 and Si_3N_4 , **Table 5**. The non-uniform load and temperature at the top surface of the plate are taken as

Table 3

Non-dimensional central deflection ($10^3 D w(0.5a, 0.5b, 0.5h)/qa^4$) of a uniformly loaded homogeneous square thin plate on Winkler-Pasternak foundations ($a = b = 100h$, $\nu = 0.3$).

	k_w	k_p	$N_x \times N_y \times N_z$				Ref. [85]	Ref. [86]
			$7 \times 7 \times 7$	$9 \times 9 \times 9$	$11 \times 11 \times 11$	$13 \times 13 \times 13$		
SSSS	1	1	3.8492	3.8548	3.8549	3.8549	3.8549	3.8546
		3^4	0.757	0.7629	0.763	0.763	0.763	0.763
		5^4	0.1132	0.1154	0.1152	0.1153	0.1153	0.1153
SCSC	3^4	1	1.6859	1.6802	1.6961	1.6938	1.6986	1.7
		3^4	0.5697	0.5705	0.576	0.5743	0.5761	0.576
		5^4	0.1011	0.1058	0.1055	0.1052	0.1057	0.106
SF SF	5^4	1	1.6578	1.6578	1.6586	1.6589	1.6591	1.66
		3^4	0.8507	0.8533	0.853	0.8525	0.8523	0.851
		5^4	0.201	0.2032	0.2034	0.2034	0.2032	0.203

Table 4

Non-dimensional deflection and stress of uniformly loaded FGM rectangular plate with simply supported edges on elastic foundation ($b = 3a = 30h$).

n_z	K_0	J_0	w^*	σ_x^*	σ_y^*	σ_{xy}^*	
0	0	7 \times 7 \times 7	1.2416	0.7052	0.231	0.267	
			1.2554	0.7155	0.2461	0.283	
			1.2547	-0.7155	-0.2444	0.2855	
			1.2546	0.715	0.2443	0.286	
			1.2545	0.7153	0.2445	0.286	
			Ref. [87]	1.2583	0.716	0.2447	0.289
		9 \times 9 \times 9	1.2097	0.6863	0.2238	0.262	
			1.2234	0.6965	0.2389	0.2781	
			1.2227	0.6965	0.2372	0.2806	
			1.2226	0.6961	0.2371	0.2811	
100	0	15 \times 15 \times 15	1.2226	0.6963	0.2372	0.2811	
			Ref. [87]	1.226	0.6969	0.2375	0.284
			7 \times 7 \times 7	1.1508	0.6516	0.2107	0.2526
			9 \times 9 \times 9	1.1643	0.6618	0.2258	0.2687
			11 \times 11 \times 11	1.1637	0.6617	0.2241	0.2712
		100	13 \times 13 \times 13	1.1635	0.6613	0.224	0.2717
			15 \times 15 \times 15	1.1635	0.6615	0.2241	0.2717
			Ref. [87]	1.1662	0.6618	0.2245	0.2744
			7 \times 9 \times 9	1.1232	0.6352	0.2045	0.2483
			9 \times 9 \times 9	1.1366	0.6453	0.2196	0.2644
0	100	100	11 \times 11 \times 11	1.1359	0.6453	0.2179	0.2669
			13 \times 13 \times 13	1.1358	0.6449	0.2178	0.2674
			15 \times 15 \times 15	1.1358	0.6451	0.2179	0.2674
			Ref. [87]	1.1382	0.6452	0.2183	0.27
			7 \times 7 \times 7	2.494	0.3211	0.1052	0.1208
		0	9 \times 9 \times 9	2.5136	0.3247	0.1117	0.1278
			11 \times 11 \times 11	2.5109	0.3245	0.1109	0.1289
			13 \times 13 \times 13	2.5107	0.3243	0.1108	0.1291
			15 \times 15 \times 15	2.5106	0.3244	0.1109	0.1291
			Ref. [87]	2.5134	0.325	0.1111	0.1306
1	0	100	7 \times 7 \times 7	2.3694	0.3042	0.0987	0.1164
			9 \times 9 \times 9	2.3895	0.3078	0.1052	0.1234
			11 \times 11 \times 11	2.3869	0.3076	0.1044	0.1245
			13 \times 13 \times 13	2.3866	0.3074	0.1043	0.1248
			15 \times 15 \times 15	2.3866	0.3075	0.1044	0.1248
		0	Ref. [87]	2.3875	0.308	0.1047	0.1262
			7 \times 7 \times 7	2.154	0.2751	0.0876	0.1085
			9 \times 9 \times 9	2.1749	0.2789	0.0942	0.1156
			11 \times 11 \times 11	2.1724	0.2787	0.0934	0.1167
			13 \times 13 \times 13	2.1723	0.2785	0.0933	0.117
100	100	0	15 \times 15 \times 15	2.1722	0.2786	0.0934	0.117
			Ref. [87]	2.1703	0.2791	0.094	0.1182
			7 \times 7 \times 7	2.0592	0.2623	0.0827	0.1051
		100	9 \times 9 \times 9	2.0803	0.2661	0.0893	0.1122
			11 \times 11 \times 11	2.0779	0.2659	0.0885	0.1134
			13 \times 13 \times 13	2.0777	0.2657	0.0884	0.1136
		Ref. [87]	15 \times 15 \times 15	2.0777	0.2658	0.0885	0.1136
			Ref. [87]	2.0746	0.2663	0.0893	0.1148

$$q(\zeta, \eta, h) = -q_c \sin\left(\frac{\pi}{a}\zeta\right) \sin\left(\frac{\pi}{b}\eta\right) \quad (16)$$

$$T(\zeta, \eta, h) = 300 + (T_c - 300) \times \sin\left(\frac{\pi}{a}\zeta\right) \sin\left(\frac{\pi}{b}\eta\right)$$

and $T_0 = 300K$. The other thermal boundary conditions are

$$T(\zeta, \eta, 0) = T(0, \eta, z) = T(a, \eta, z) = T(\zeta, 0, z) = T(\zeta, b, z) = 300K \quad (17)$$

First, the influence of various power law exponents for a 3D-FGM skew plate is investigated. Through the thickness distribution of the transverse deflection and normal stress σ_x of central section of the skew plate ($\zeta = \frac{a}{2}, \eta = \frac{b}{2}$) are shown in Figs. 6–9. The boundary condition is assumed to be SSSS and $a = b = 10h$.

Variation of σ_x in the thickness direction under thermal loading for different combinations of material indices are shown in Fig. 6. The material variation over thickness direction has more effect on stress behavior compared to other two directions. The similar condition can be observed for mechanical loading, Fig. 8. On the other hand, the material variation in in-plane directions is the main factor for the variation of deflection of the plate under thermal loading. The material change in the thickness direction reduces the deflection. But, for the plate under mechanical loading, the main

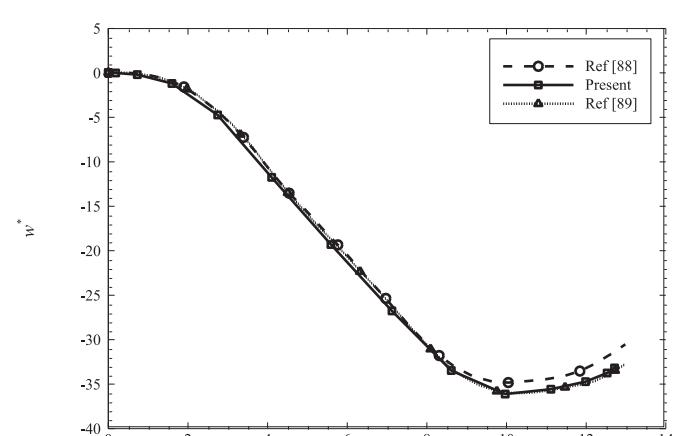


Fig. 4. Temporal evaluation of center deflection of a simply supported square plate under suddenly applied uniform loading.

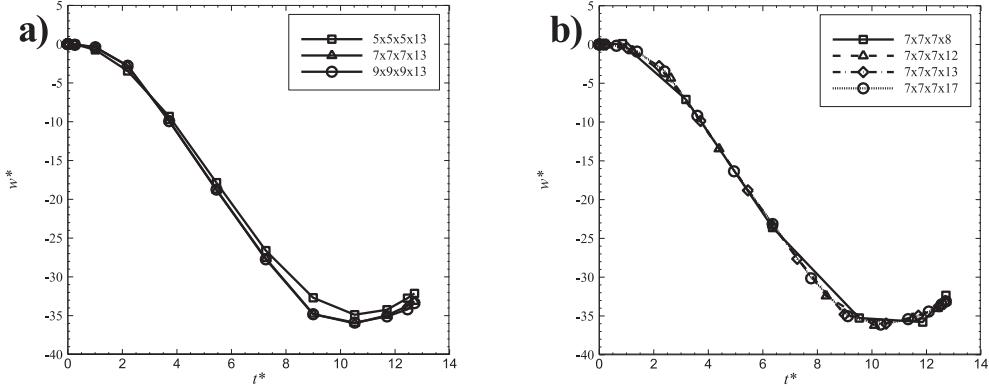


Fig. 5. Convergence study for dynamic response of the plate, (a) in space domain, (b) in time domain.

Table 5

The temperature-dependent material properties of Si_3N_4 and SUS304 [78].

Material	Property	Q_{-1}	Q_0	Q_1	Q_2	Q_3
Si_3N_4	$E(\text{Pa})$	0	348.43×10^9	-3.07×10^{-4}	2.16×10^{-7}	-8.946×10^{-11}
	ν	0	0.24	0	0	0
	$\alpha(1/\text{K})$	0	5.87×10^{-6}	9.095×10^{-4}	0	0
	$\rho(\text{kg}/\text{m}^3)$	0	2370	0	0	0
	$K(\text{W}/\text{mK})$	0	9.19	0	0	0
	$c(\text{J}/\text{kgK})$	0	0.17	0	0	0
SUS304	$E(\text{Pa})$	0	201.04×10^9	3.079×10^{-4}	-6.534×10^{-7}	0
	ν	0	0.3262	-2.002×10^{-4}	3.797×10^{-7}	0
	$\alpha(1/\text{K})$	0	12.33×10^{-6}	8.086×10^{-4}	0	0
	$\rho(\text{kg}/\text{m}^3)$	0	8166	0	0	0
	$K(\text{W}/\text{mK})$	0	12.04	0	0	0
	$c(\text{J}/\text{kgK})$	0	0.08	0	0	0

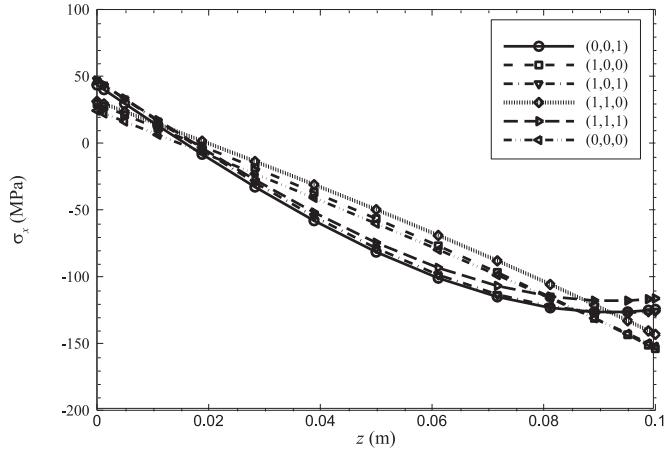


Fig. 6. Effect of power law exponents on variation of σ_x through z direction of simply supported 3D-FGM skew plate under thermal loading ($T_c = 400 \text{ K}$, $q_c = 0$, $\theta = 30^\circ$, $J_0 = K_0 = 1000$).

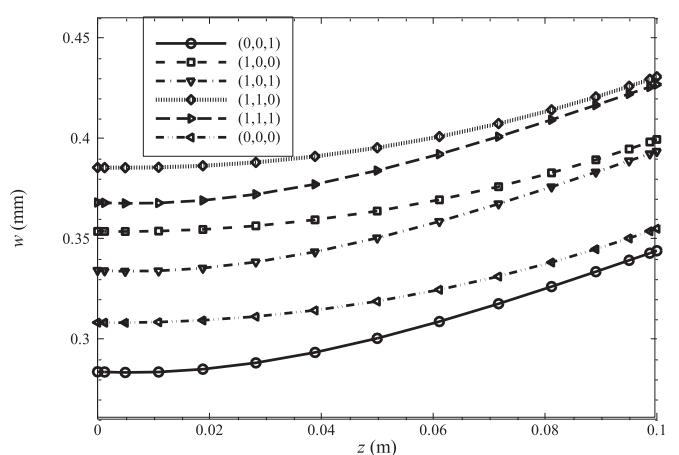


Fig. 7. Effect of power law exponents on variation of deflection through thickness of simply supported 3D-FGM skew plate under thermal loading ($T_c = 400 \text{ K}$, $q_c = 0$, $\theta = 30^\circ$, $J_0 = K_0 = 1000$).

factor of differences in deflections is the material variation through thickness direction, Fig. 9. In this case, the material variation in the in-plane directions can also increase the deflection. It is seen that the stresses and deflections of the plate strongly are affected by the directions of material variation. The obtained patterns for stresses and deflections can help designers to predict and optimize the behavior of structures.

Figs. 10–13 show the effect of boundary conditions on σ_x and the

center deflection of the plate. It is observed that in both thermal and mechanical loading, the deflection decreases by increasing the constraints of the plate boundaries. As it shown in Fig. 12, the neutral plane is independent of boundary conditions of the skew plate under mechanical loading. Comparing Figs. 10 and 12, it reveals that the effect of boundary conditions on the stress is different for the plates under mechanical or thermal loading, e.g. the FCFC or SSSS cases may have higher values of stresses, see Figs. 10 and 12.

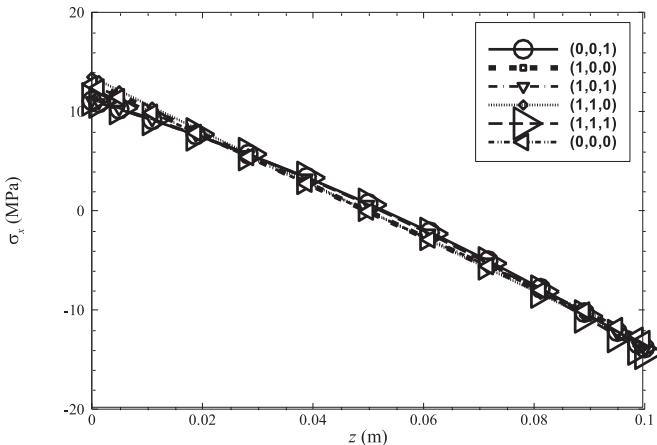


Fig. 8. Effect of power law exponents on variation of σ_x through z direction of simply supported 3D-FGM skew plate under mechanical loading ($T_c = 300$ K, $q_c = 1$ MPa, $\theta = 30^\circ$, $J_0 = K_0 = 1000$).

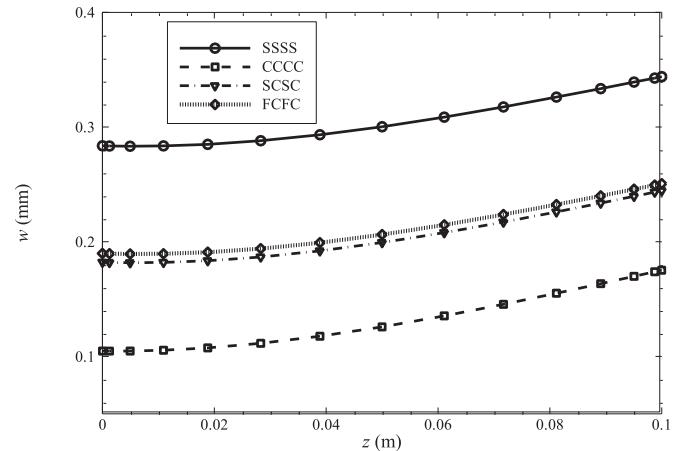


Fig. 11. Variation of deflection through thickness of 1D-FGM plates under thermal loading for various boundary conditions ($T_c = 400$ K, $q_c = 0$, $\theta = 30^\circ$, $J_0 = K_0 = 1000$, $n_\eta = n_\zeta = 0$, $n_z = 1$).

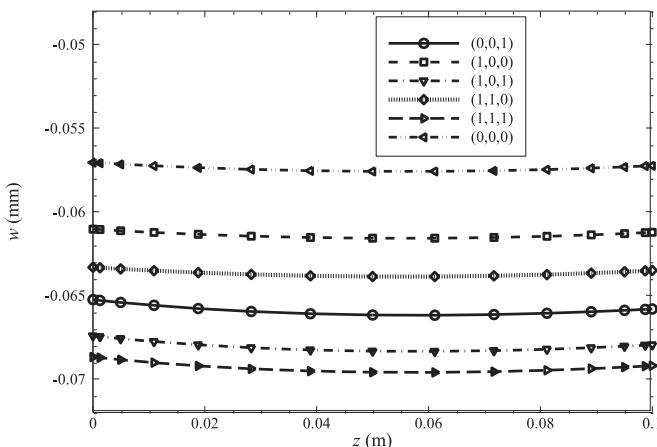


Fig. 9. Effect of power law exponents on variation of deflection through thickness of simply supported 3D-FGM skew plate under mechanical loading ($T_c = 300$ K, $q_c = 1$ MPa, $\theta = 30^\circ$, $J_0 = K_0 = 1000$).

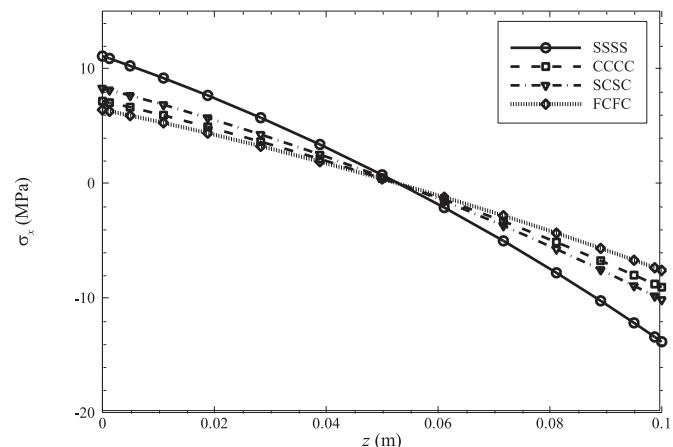


Fig. 12. Variation of σ_x through thickness of 1D-FGM plates under mechanical loading for various boundary conditions ($T_c = 300$ K, $q_c = 1$ MPa, $\theta = 30^\circ$, $J_0 = K_0 = 1000$, $n_\eta = n_\zeta = 0$, $n_z = 1$).

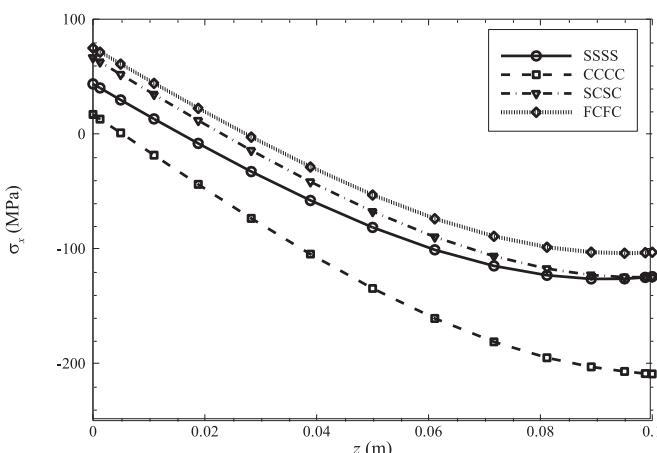


Fig. 10. Variation of σ_x through thickness of 1D-FGM plates under thermal loading for various boundary conditions ($T_c = 400$ K, $q_c = 0$, $\theta = 30^\circ$, $J_0 = K_0 = 1000$, $n_\eta = n_\zeta = 0$, $n_z = 1$).

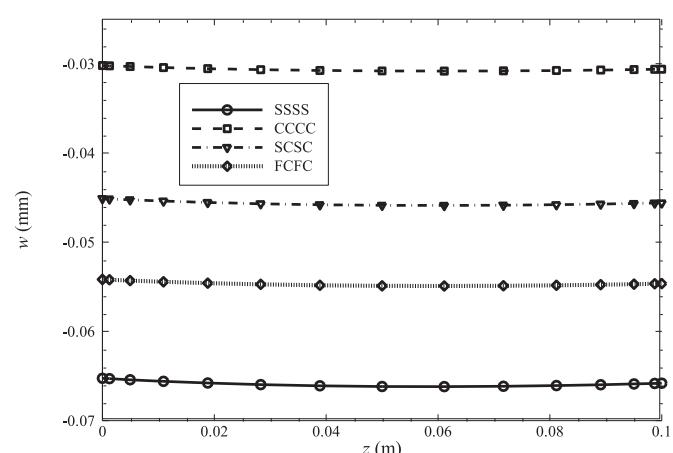


Fig. 13. Variation of deflection through thickness of 1D-FGM plates under mechanical loading for various boundary conditions ($T_c = 300$ K, $q_c = 1$ MPa, $\theta = 30^\circ$, $J_0 = K_0 = 1000$, $n_\eta = n_\zeta = 0$, $n_z = 1$).

The increase of temperature at the top of the plate increases the σ_x and deflection as it was expected, Figs. 14 and 15.

In Figs. 16–19, the effects of the change of skew angle θ on stress and deflection are investigated. In the case of mechanical loading,

the stress σ_x on the both top and bottom of the plate, decreases by increasing the angle, Fig. 16. Moreover, the deflection decreases by increasing the angle, Fig. 17. But the deflection curve trend of the plate under the thermal loading has a different behavior, Fig. 19.

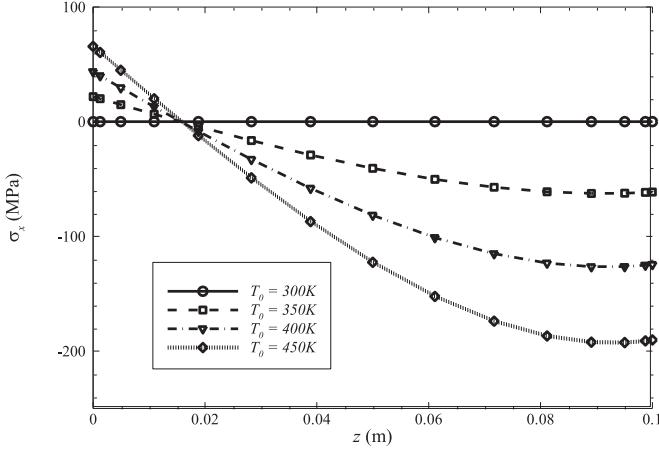


Fig. 14. Effect of thermal condition on variation of σ_x through z direction of simply supported 1D-FGM plates ($q_c = 0, \theta = 30^\circ, J_0 = K_0 = 1000, n_\eta = n_\zeta = 0, n_z = 1$).

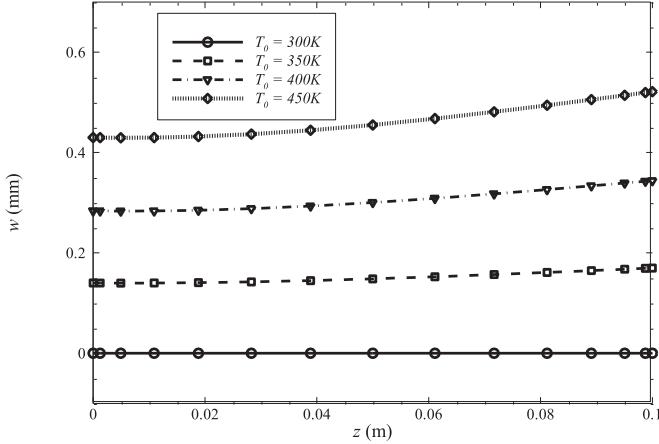


Fig. 15. Effect of thermal condition on variation of deflection through z direction of simply supported 1D-FGM plates ($q_c = 0, \theta = 30^\circ, J_0 = K_0 = 1000, n_\eta = n_\zeta = 0, n_z = 1$).

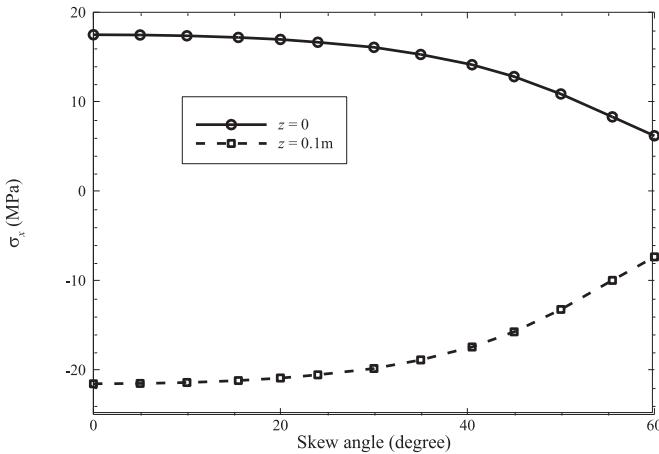


Fig. 16. Effect of skew angle on the normal stress of a 1D-FGM skew plate under mechanical loading ($T_c = 300 K, q_c = 1MPa, J_0 = K_0 = 1000, n_\eta = n_\zeta = 0, n_z = 1$).

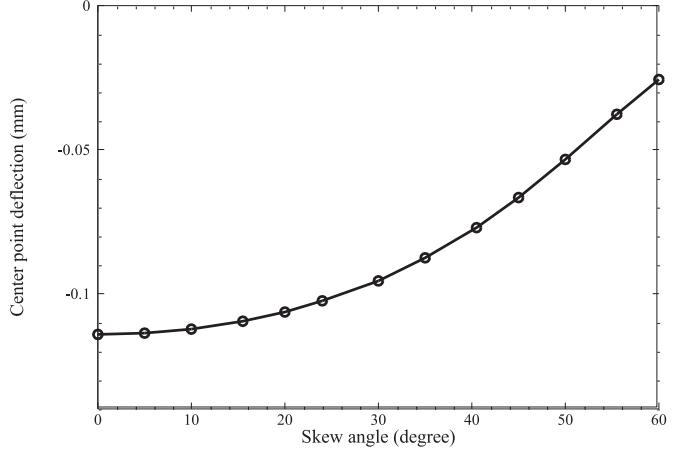


Fig. 17. Effect of skew angle on the deflection of a 1D-FGM skew plate under mechanical loading ($T_c = 300 K, q_c = 1MPa, J_0 = K_0 = 1000, n_\eta = n_\zeta = 0, n_z = 1$).

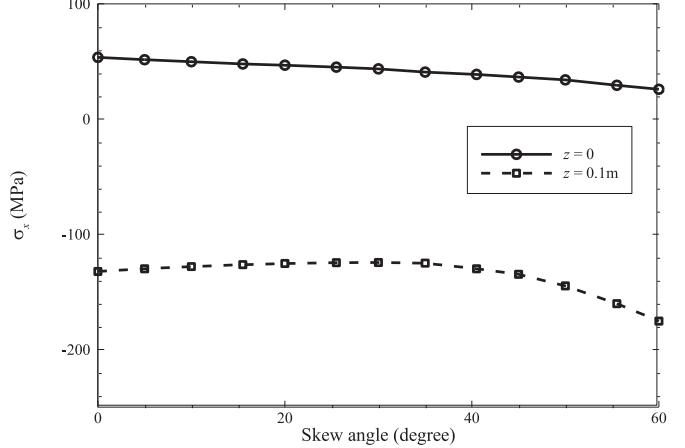


Fig. 18. Effect of skew angle on the normal stress of a 1D-FGM skew plate under thermal loading ($T_c = 400 K, q_c = 0, J_0 = K_0 = 1000, n_\eta = n_\zeta = 0, n_z = 1$).

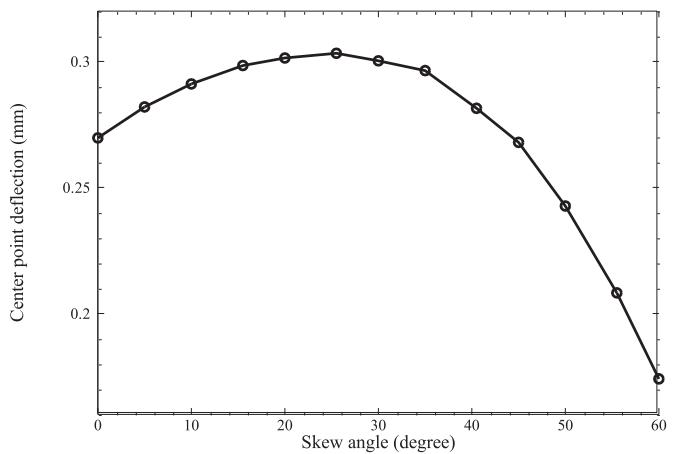


Fig. 19. Effect of skew angle on the deflection of a 1D-FGM skew plate under thermal loading ($T_c = 400 K, q_c = 0, J_0 = K_0 = 1000, n_\eta = n_\zeta = 0, n_z = 1$).

The stress σ_x in ($\zeta = \frac{a}{2}, \eta = \frac{b}{2}, z = 0$) decreases by increasing the angle similar to the plate under the mechanical loading although the stress in ($\zeta = \frac{a}{2}, \eta = \frac{b}{2}, z = h$) has a dissimilar behavior. Fig. 19 shows that there is an angle in which the maximum deflection of center point occurs for the simply supported skew plate on elastic foundation under thermal loading which does not happen for the case of mechanical loading, Fig. 17.

In Figs. 20–23, the effects of foundation parameters on the amount of stress and deflection are investigated. The figures indicate that in the region of analysis both σ_x and deflection are more strongly dependent upon J_0 rather than K_0 under thermal and mechanical loadings.

Figs. 24–27 show the temporal evaluation of central deflection of a multi-directional FGM skew plate under suddenly applied loading. The material variation in each direction increases the maximum deflection and its corresponding time which is due to the resulting decrease in the modulus of elasticity, Fig. 24. It is also observed that the effect of material variation in z direction on deflection history of the plate is more than that of the in-plane directions. This figure shows that one can change the transient response of a structure only with material varying in some directions and without any change in the geometry and thickness of the structure. It is an important issue especially for the aerodynamic force when the structures

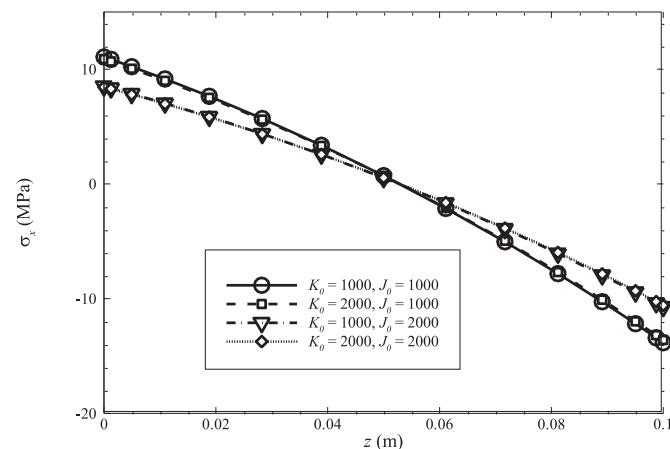


Fig. 20. Effect of foundation parameters on the normal stress of a 1D-FGM skew plate under mechanical loading ($T_c = 300$ K, $\theta = 30^\circ$, $q_c = 1$ MPa, $n_\eta = n_\zeta = 0$, $n_z = 1$).

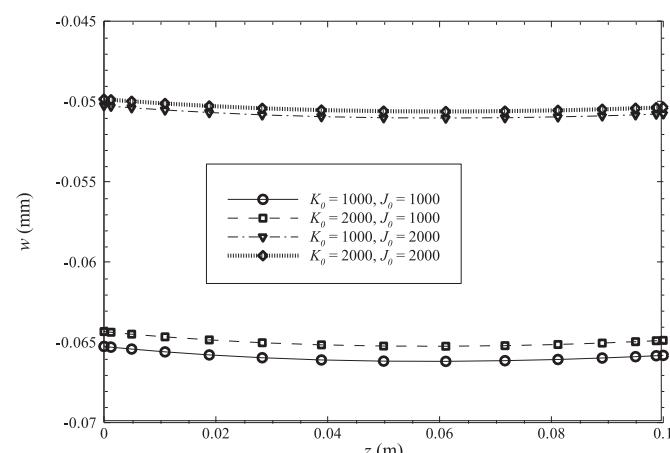


Fig. 21. Effect of foundation parameters on the deflection of a 1D-FGM skew plate under mechanical loading ($T_c = 300$ K, $\theta = 30^\circ$, $q_c = 1$ MPa, $n_\eta = n_\zeta = 0$, $n_z = 1$).

are designed for a special geometry and any change may cause disturbance in its performance.

It is also seen that in the region of analysis the effect of the shearing layer elastic parameter (J_0) in transient response of a FGM

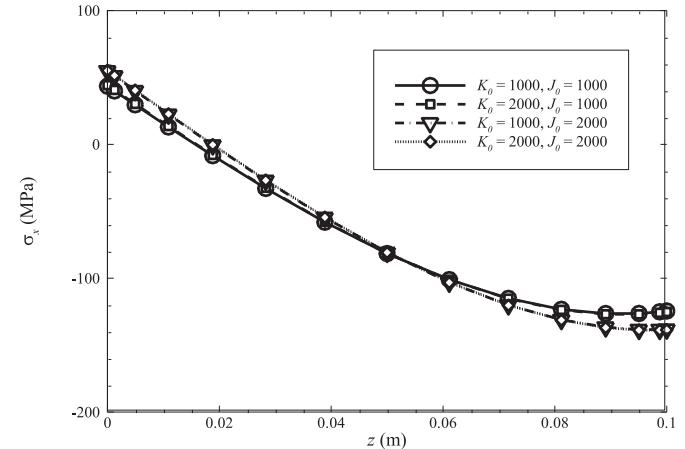


Fig. 22. Effect of foundation parameters on the normal stress of a 1D-FGM skew plate under thermal loading ($T_c = 400$ K, $\theta = 30^\circ$, $q_c = 0$, $n_\eta = n_\zeta = 0$, $n_z = 1$).

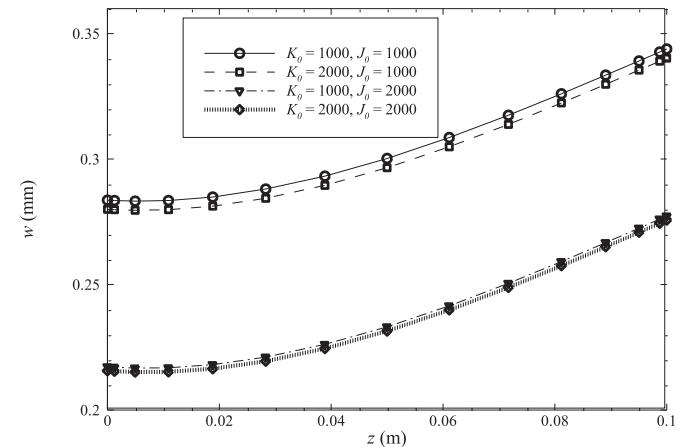


Fig. 23. Effect of foundation parameters on the deflection of a 1D-FGM skew plate under thermal loading ($T_c = 400$ K, $\theta = 30^\circ$, $q_c = 0$, $n_\eta = n_\zeta = 0$, $n_z = 1$).

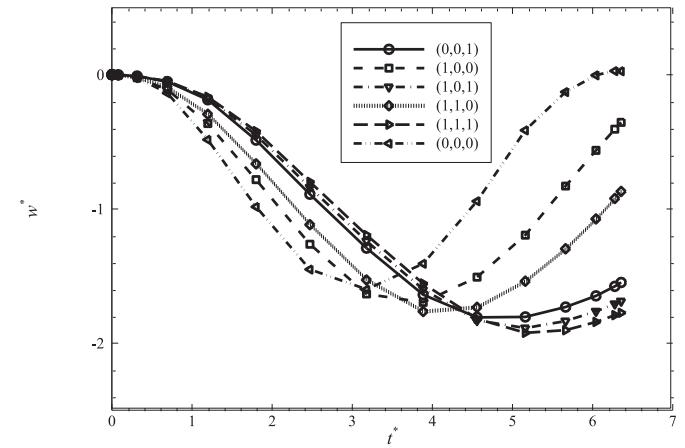


Fig. 24. Effect of grading exponents on the time history of deflection of a 1D-FGM skew plate under suddenly applied loading ($T_c = 300$ K, $\theta = 30^\circ$, $q_c = 1$ MPa, $J_0 = K_0 = 1000$, $n_\eta = n_\zeta = 0$, $n_z = 1$).

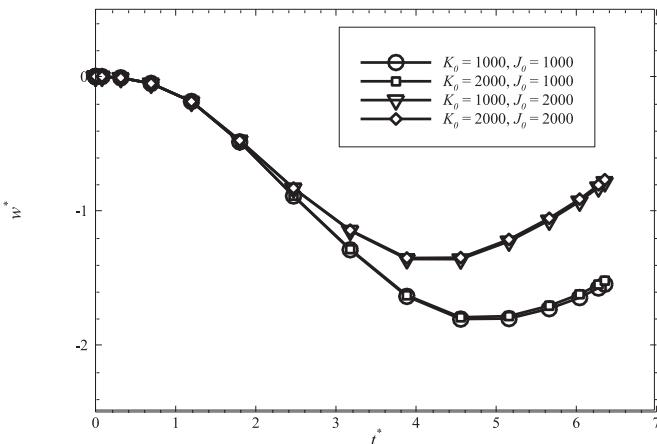


Fig. 25. Effect of foundation parameters on the time history of deflection of a 1D-FGM skew plate under suddenly applied loading ($T_c = 300 \text{ K}$, $\theta = 30^\circ$, $q_c = 1 \text{ MPa}$, $n_\eta = n_\zeta = 0$, $n_z = 1$).

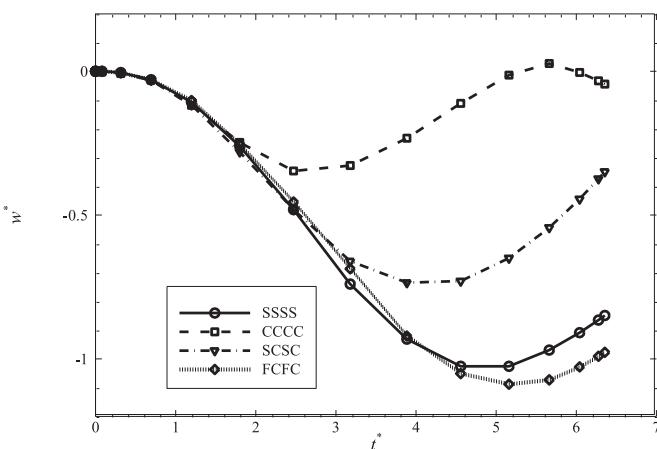


Fig. 26. Effect of boundary conditions on the time history of deflection of a 1D-FGM skew plate under suddenly applied loading ($T_c = 300 \text{ K}$, $\theta = 45^\circ$, $q_c = 1 \text{ MPa}$, $J_0 = K_0 = 1000$, $n_\eta = n_\zeta = 0$, $n_z = 1$).

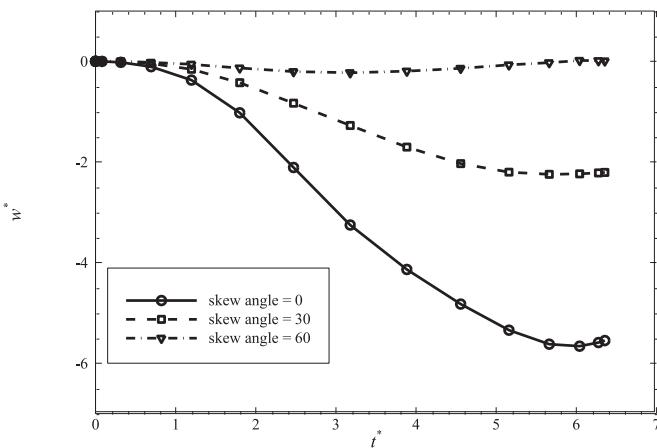


Fig. 27. Effect of skew angle on the time history of deflection of a 1D-FGM skew plate under suddenly applied loading ($T_c = 300 \text{ K}$, $q_c = 1 \text{ MPa}$, $J_0 = K_0 = 1000$, $n_\eta = n_\zeta = 0$, $n_z = 1$).

skew plate under mechanical loading is more than that of the Winkler foundation parameter (K_0). Moreover, the increase of J_0 and K_0 reduce the maximum deflection and its corresponding time

which is due to the increase of structure stiffness, Fig. 25. As expected, rising the constraints of boundaries reduces the maximum deflection and the time to reach it, Fig. 26. Moreover, it is seen that the increase of angle θ reduces the maximum deflection of transient response of FGM skew plate and its corresponding time, Fig. 27.

7. Conclusions

In this paper, three dimensional thermo-mechanical analysis of a 3D-FGM skew plate on elastic foundation was investigated for the first time. In this regard, their equations of the three dimensional theory of elasticity were derived and solved by using DQM. Additionally, 4D DQM was used to solve the equations of dynamic response of a 3D-FGM skew plate on elastic foundation. The influence of different parameters including foundation parameters, power law components and various boundary conditions on the bending behavior of FGM skew plate were presented. It is found that the direction of material gradation in a FGM skew plate has a noticeable effect on the behavior of the plate. The results also show that the power law index and the direction of material change can be used as effective parameters to design the rectangular and skew plate structures properly. It would be more important when the behavior of the structure can be enhanced with minor modification of material gradation direction without any change in the pre-design geometry of the structure obtained based on other parameters such as space limitations or aerodynamic forces. The results of the current study are determined by 3D theory of elasticity and can be employed as benchmark for other studies.

Appendix.1

Variable changes used for transforming Cartesian coordinates to skew coordinates are as follows

$$x = \zeta + \eta \sin \theta$$

$$y = \eta \cos \theta$$

$$u_\zeta = u \cos \theta - v \sin \theta$$

$$v_\eta = u \sin \theta + v \cos \theta$$

$$\frac{\partial}{\partial x} = \frac{\partial}{\partial \zeta}$$

$$\frac{\partial}{\partial y} = \frac{1}{\cos \theta} \frac{\partial}{\partial \eta} - \tan \theta \frac{\partial}{\partial \zeta}$$

$$\frac{\partial^2}{\partial x^2} = \frac{\partial^2}{\partial \zeta^2}$$

$$\frac{\partial^2}{\partial y^2} = \frac{1}{\cos^2 \theta} \frac{\partial^2}{\partial \eta^2} - 2 \frac{\tan \theta}{\cos \theta} \frac{\partial^2}{\partial \eta \partial \zeta} + \tan^2 \theta \frac{\partial^2}{\partial \zeta^2}$$

$$\frac{\partial^2}{\partial x \partial y} = \frac{1}{\cos \theta} \frac{\partial^2}{\partial \zeta \partial \eta} - \tan \theta \frac{\partial^2}{\partial \zeta^2}$$

$$\frac{\partial^2}{\partial x \partial z} = \frac{\partial^2}{\partial \zeta \partial z}$$

$$\frac{\partial^2}{\partial y \partial z} = \frac{1}{\cos \theta} \frac{\partial^2}{\partial z \partial \eta} - \tan \theta \frac{\partial^2}{\partial z \partial \zeta}$$

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